

A. DESIGN DATA:

1. ENVIRONMENTAL DESIGN LOADS:

SNOW:(1/50)	$S_S = 0.0143 \times Z = 9.55 \text{ kPa}$	$I_E = 1.0$	Cs:1.00	
	$S_R = 0.66 + 0.00033 \times Z = 0.89 \text{ kPa}$	$C_b = 0.80$		
	$Z = 668 \text{ MTS}$			
	$S = I_S [S_S (C_b, C_w, C_s, C_e) + S_r]$	$S = 8.53 \text{ kPa}$		
BCBC 2024 CODE:				
	$S_a (0.2) = 0.43$	SITE CLASS "C"		
	$S_a (0.5) = 0.37$	$M_v = 1.0$		
	$S_a (1.0) = 0.23$			
	$S_a (2.0) = 0.15$	$I_E = 1.0$		
	$PGA = 0.20$			
	$Q(50): 0.32 \text{ Kpa}$			

- 1.1 DESIGN AND CONSTRUCTION STANDARDS ARE TO BE IN ACCORDANCE WITH GENERAL INTENT OF THE BCBC 2024 BUILDING BY-LAW.
- 1.2 NEW CONSTRUCTION SHALL NOT LESSEN EXISTING CONDITIONS AND PERFORMANCE FOR FIRE PREVENTION AND LIFE SAFETY.
- 1.3 NEW MATERIAL S AND METHODS SHALL MEET BCBC 2024 REQUIREMENTS AND HAVE EQUAL OR BETTER PERFORMANCE THAN EXISTING CONDITIONS FOR FIRE PREVENTION AND LIFE SAFETY.
- 1.4 ALL NEW NON-STRUCTURAL COMPONENTS AND EQUIPMENT SHALL MEET BCBC 2024 REQUIREMENT.

B. FOUNDATION:

1. FOUNDATION AS PER GEOTECHNICAL REPORT BY TBD.
2. BEARING SURFACES MUST BE APPROVED BY THE GEOTECHNICAL ENGINEER BEFORE PROCEEDING WITH WORK
3. FOUNDATION DESIGN BEARING CAPACITY: 2500 PSF ALLOWABLE BEARING PRESSURE  
3750 PSF FACTORED ULTIMATE BEARING RESISTANCE  
TO BE CONFIRMED
4. REFER TO GEOTECHNICAL REPORT FOR OTHER SPECIFIC DESIGN REQUIREMENTS FOR FOOTINGS, SOIL SLOPES, FROST PROTECTION. MINIMUM COVER. ETC.
5. UNLESS OTHERWISE SHOWN, CENTER FOOTINGS UNDER COLUMNS.
6. PROVIDE 50mm GROUND SEAL UNDER FOOTINGS AS REQUIRED BY SOIL CONDITIONS.
7. FOR GROUND ELEVATIONS AND DRAINAGE SLOPES, SEE ORIGINAL EXISTING AND ARCHITECT'S DRAWINGS.
8. FOOTINGS MAY HAVE TO BE LOWERED TO ACCOMMODATE MECHANICAL OR ELECTRICAL SERVICES. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR ELEVATIONS. FOOTINGS ARE NOT TO BE UNDERMINED BY EXCAVATIONS FOR SERVICES, PITS, ETC. U.N.O. ON STRUCTURAL DRAWINGS.
9. FOOTING ELEVATIONS SHOWN ARE NOT FINAL AND MAY VARY ACCORDING TO SITE CONDITIONS. ALL FOOTINGS MUST BE TAKEN TO A BEARING LAYER APPROVED BY THE GEOTECHNICAL ENGINEER.
10. BEARING SURFACES MUST BE PROTECTED FROM FREEZING BEFORE AND AFTER FOOTINGS ARE POURED.
11. SUB-BASE DESIGN OF SOIL UNDER THE SLAB ON GRADE SHALL BE IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.
12. ALL NEW SLAB ON GRADE SHALL HAVE 6 mil POLYETHYLENE VAPOUR BARRIER ON GRANULAR FREE DRAINING FILL COMPACTED TO AT LEAST 95% MODIFIED PROCTOR DRY DENSITY (MPDD). PROVIDE SAWCUT 25mm OR 1/4 OF SLAB DEPTH, WHICHEVER IS DEEPER AT 3000 mm O/C MAXIMUM.

C. EXCAVATION, TRENCHING AND BACKFILL:

1. REFER TO GEOTECHNICAL REPORT FOR DETAILS OF EXISTING GROUND CONDITIONS.
2. PROVIDE ENGINEERS INSPECTION OF EXCAVATIONS REQUIRED BY WCB.
3. ESTABLISH LOCATION AND STATE OF USE OF BURIED UTILITIES AND STRUCTURES. MAINTAIN AND PROTECT FROM DAMAGE WATER, SEWER, GAS ELECTRIC, TELEPHONE AND OTHER UTILITIES AND STRUCTURES ENCOUNTERED.
4. PROTECT EXISTING BUILDINGS AND SURFACE FEATURES FROM DAMAGE WHILE WORK IS IN PROGRESS AND REPAIR DAMAGE RESULTING FROM WORK. PROVIDE SHORING AND UNDERPINNING REQUIRED. PROVIDE TEMPORARY ENCLOSURES FOR DUST CONTAINMENT AS REQUIRED BY THE OWNER.
5. CUT PAVEMENTS NEATLY ALONG LIMITS OF PROPOSED EXCAVATION.
6. MAINTAIN THE SITE IN A WELL DRAINED CONDITION. DO NOT OBSTRUCT THE FLOW OF SURFACE DRAINAGE OR NATURAL WATERCOURSES. DO NOT CONTAMINATE WATERCOURSES WITH SILT.
7. PROVIDE SETTLEMENT POND AND/OR FILTER FABRIC SCREEN ON DRAINAGE INLET.
8. EXCAVATE WITH CARE NEAR EXISTING FOUNDATIONS AND ENSURE THAT FOUNDATIONS OF EXISTING STRUCTURES ADJACENT TO EXCAVATED AREAS ARE NOT DAMAGED, WEAKENED OR IMPAIRED.
9. ENSURE THE BOTTOM OF EXCAVATION IS UNDISTURBED SOIL, LEVEL AND FREE OF ALL LOOSE, SOFT OR ORGANIC MATTER, AND IS PROTECTED AND KEPT DRY UNTIL THE CONCRETE IS PLACED.

THOROUGHLY COMPACT THE BASE OF THE EXCAVATION PRIOR TO FOUNDATION CONSTRUCTION, TO DENSIFY THE SOIL LOOSENED BY THE EXCAVATION EQUIPMENT. PROTECT COMPACTED BASE WITH 75mm 15MPa BLINDING LAYER. GEOTECHNICAL ENGINEER TO REVIEW EXCAVATED AREAS PRIOR TO FORMING AND PLACING BLINDING LAYER.

10. PROTECT WATERPROOFING WITH 13mm ASPHALT IMPREGNATED FIBERBOARD BEFORE BACKFILL.
11. USE HAND-OPERATED COMPACTION EQUIPMENT WITHIN 1 METER OF WALLS AND FOOTINGS.
12. DO NOT BACKFILL AROUND CONCRETE STRUCTURES WITHIN FOURTEEN (14) DAYS OF CONCRETE PLACEMENT UNLESS OTHERWISE APPROVED BY THE OWNER'S REPRESENTATIVE. HOWEVER, BACKFILLING AROUND SPREAD FOUNDATIONS MAY BE CARRIED OUT AFTER FOUR (4) DAYS.
13. PLACE BACKFILL LAYERS ON BOTH SIDES OF INSTALLED WORK TO EQUALIZE LOADING.
14. ALL BACKFILL SHALL BE CLEAN FREE DRAINING GRANULAR MATERIAL AND SHALL BE PLACED AND COMPACTED IN THIN LAYERS AS INDICATED BY THE GEOTECHNICAL ENGINEER. SOIL COMPACTION WITHIN 1m OF THE WALL TO BE ACHIEVED USING LIGHT HAND COMPACTING EQUIPMENT SUCH AS A 300mm TO 450mm PLATE TAMPER. AREAS ON CITY PROPERTY TO BE COMPACTED WITH FILL MEETING CITY SPECS AS INSTRUCTED BY GEOTECHNICAL CONSULTANT.
15. PROTECT AND MAINTAIN ALL EXISTING TREES, PLANTS AND VEGETATION.
16. PROTECT AND MAINTAIN ALL EXISTING STREET FURNISHINGS.
17. UNDERPIN EXISTING FOUNDATION IF NECESSARY.
18. CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF TEMPORARY SHORING TO EXCAVATION AND TEMPORARY UNDERPINNING TO EXISTING FOUNDATION.
19. BASED ON AVAILABLE INFORMATION THE SOIL TO BE EXCAVATED IS NOT EXPECTED TO BE CONTAMINATED. HOWEVER, IF THE SOIL OR A PART OF IT IS FOUND TO BE CONTAMINATED, THE CONTRACTOR SHALL INFORM THE OWNER'S REPRESENTATIVE IMMEDIATELY, AND CONDUCT THEMSELVES ACCORDING TO CONTAMINATED SITE REGULATIONS AND AS PER THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.

D. CAST-IN-PLACE CONCRETE:

1. PRODUCTS:

CEMENT	GU
AGGREGATES	NORMAL DENSITY
VAPOR MEMBRANE	6 MIL POLYETHYLENE SHEETING
JOINT SEALANT/JOINT FILLER	SYNTHETIC RUBBER-BASED TWO COMPONENT BITUMINOUS IMPREGNATED FIBER BOARD IN ACCORDANCE WITH ASTM D1751

DO NOT USE ADMIXTURES CONTAINING CALCIUM CHLORIDE.

1.1 MIX DESIGN (U.N.O.)

LOCATION	MIN 28 DAYS STRENGTH (MPa)	MAX WATER/ CEMENT RATIO	SLUMP (mm)	MAX. AGG. SIZE (mm)	EXPOSURE CLASS	AIR CONTENT %
FOOTINGS AND RETAINING WALLS	30	0.55	80±20	20	F2	4 TO 7
INTERIOR SLAB ON GRADE	25	-	80±20	20	N	-

2. EXECUTION:

- 2.1 LOCATE JOINTS AS SHOWN ON THE DRAWINGS AND STANDARDS AND PROVIDE CONTROL JOINTS AS REQUIRED BY THE ARCHITECT. DO NOT EXCEED 6M SPACING BETWEEN CONTROL JOINTS IN GRADE SLABS.
- 2.2 BEFORE POURING CONCRETE, FIX ANCHOR BOLTS SECURELY BY WIRING THE BOTTOMS TO REINFORCING STEEL AND SECURING THE TOPS WITH TEMPLATES. SET ANCHOR BOLTS PERPENDICULAR TO THE BEARING SURFACE AND PROTECT THE THREADS. AFTER POURING CONCRETE, ENSURE NUTS TURN FREELY.
- 2.3 FINISH ALL SURFACES AS REQUIRED BY THE SPECIFICATIONS OR AS NOTED ON THE ARCHITECTURAL DRAWINGS. CURE AND FINISH THE CONCRETE TO SUIT THE INTENDED USE OF THE SURFACE. GRIND EXPOSED SHARP EDGES OF CONCRETE. IF FLOOR FINISHES ARE NOT OTHERWISE SPECIFIED, PROVIDE THE FOLLOWING:  

CONCRETE FLOORS: STEEL TROWEL  
WET AREAS: STEEL TROWEL THEN BROOM
- 2.4 CONCRETE TO BE MIX, PLACED AND MOIST CURED AS ACCORDING TO CSA A23.1-09.
- 2.5 ALL EXPOSED CONCRETE CORNERS TO HAVE 20mm CHAMFER.
- 2.6 BEFORE POURING CONCRETE, ENSURE THAT ALL EMBEDDED ITEMS, SUCH AS ANCHOR BOLTS, SLEEVES AND WATER STOPS ARE IN PLACE TO THE SATISFACTION OF THE ENGINEER.
- 2.7 SUBMIT PROPOSED CONCRETE MIX DESIGNS FOR APPROVAL TO THE ENGINEER AND TESTING AGENCY RETAINED BY THE CONTRACTOR, EACH MIX DESIGN IS TO HAVE ITS AREAS OF USE CLEARLY IDENTIFIED. CONTRACTOR TO SUBMIT TO THE ENGINEER WRITTEN DOCUMENTATION CONFIRMING THAT EACH AGGREGATE SOURCE TO BE USED ON THE PROJECT WILL COMPLY WITH CSA A23.1-CLAUSE 5.5 ON ALKALI - REACTIVITY OF AGGREGATES AND OTHER REACTIONS.
- 2.8 CONCRETE QUALITY IS TO BE TESTED BY THE TESTING AGENCY ACCORDING TO CSA-A23.2-09 INCLUDING STRENGTH, AND SLUMP TESTS, AIR CONTENT AND W/C RATIOS FOR EACH CONCRETE POUR, WITH REPORTS SUBMITTED TO THE STRUCTURAL ENGINEER.
- 2.9 THE CONTRACTOR SHALL NOT PROCEED WITH PLACING CONCRETE THAT FAILS TO MEET REQUIREMENTS. IF CONCRETE TESTS SHOW CONCRETE TO BE LESS THAN REQUIRED QUALITY, THE ENGINEER SHALL HAVE THE RIGHT TO HAVE MIX DESIGN ALTERED FOR THE REMAINDER OF THE WORK AT NO EXTRA COST TO THE OWNER. FURTHER TESTS AND REMEDIAL MEASURES

- 2.10 MAY BE REQUIRED AS NOTED IN CLAUSE 4.4.6.7.1 OF CSA-23.1. WHEN REQUIRED, CORE TEST SHALL BE CONDUCTED IN ACCORDANCE WITH CSA-23.1 CLAUSE 4.4.6.6.2. THE EXPENSE OF SUCH WORK SHALL BE PAID BY THE CONTRACTOR  
VERTICAL DROP OF CONCRETE NOT TO EXCEED 1500mm.
- 2.11 COMPACT AND CONSOLIDATE CONCRETE WITH INTERNAL VIBRATORS TO SIZE, SPACING, ETC. IN CSA A23.1 CLAUSE 19.4. WORK CONCRETE AROUND ALL EMBEDDED MATERIAL AND INTO PREVIOUSLY PLACED CONCRETE LIFT.
- 2.12 WHEN TEMPERATURE IS BELOW 4°C PROVIDE HEAT AND PROTECTION IN ACCORDANCE WITH CSA-A23.1-04, CLAUSE 7.4.1.5. WHEN TEMPERATURE IS 27°C OR ABOVE, PROTECT IN ACCORDANCE WITH CLAUSE 7.4.1.2 AND 7.4.1.4.
- 2.13 CURE AND PROTECT CONCRETE IN ACCORDANCE WITH CSA-A23.1-09.
- 2.14 OPENINGS, PIPE SLEEVES, ETC. IN STRUCTURAL CONCRETE ARE NOT PERMITTED EXCEPT AS SPECIFICALLY APPROVED BY THE ENGINEER.

E. CONCRETE REINFORCEMENT:

1. PRODUCTS:

REINFORCING STEEL:	GRADE 400W DEFORMED BARS TO CSA G30.18M-09
WELDED STEEL WIRE FABRIC:	GRADE 400 TO CSA G30.3-M83 (R1998)
PROTECTIVE COATING (IF SHOWN):	EPOXY

DO NOT USE MATERIALS WITH LOOSE SCALY RUST, DIRT, OIL, PAINT OR OTHER BOND-BREAKING COATINGS.

2. EXECUTION:

- 2.1 DO NOT FIELD BEND REINFORCEMENT EXCEPT WHERE AUTHORIZED BY THE ENGINEER.
- 2.2 IF LOCATION IS NOT SPECIFICALLY NOTED ON THE DRAWINGS, PLACE REINFORCING STEEL SYMMETRICALLY WITH RESPECT TO SUPPORTS FOR MID-SPAN.
- 2.3 CLEAN ALL REINFORCING STEEL BEFORE PLACING CONCRETE.
- 2.4 PROVIDE THE FOLLOWING MINIMUM CONCRETE COVER FOR REINFORCEMENT:

CONCRETE SURFACE IS EXPOSED TO GROUND OR WEATHER	CONCRETE IS PLACED AGAINST GROUND	75mm
	CONCRETE SHEAR WALL	30mm
	CONCRETE IS PLACED AGAINST FORMWORK OR LEAN MIX	50mm
CONCRETE SURFACE NOT EXPOSED TO GROUND OR WEATHER	BEAMS, COLUMNS, PADS AND PIERS	40mm
	CONCRETE SHEAR WALL	20mm OR BAR DIA.
	SLABS ON GRADE (TOP STEEL)	50mm

- 2.5 REINFORCEMENT TO BE DETAILED AND FABRICATED AS ACCORDING TO CSA A23.3-04 APPENDIX A.
- 2.6 ALL TIES TO BE 135° UNLESS NOTED OTHERWISE.
- 2.7 SPLICES ARE STAGGERED SO THAT NO MORE THAN 50% OF EACH INDIVIDUAL LAYER OF REINFORCEMENT IS SPLICED AT ANY CROSS SECTION, U.N.O.
- 2.8 LAP SPlice LENGTH OF REINFORCEMENT BASED ON LARGER BAR BEING SPLICED, UNLESS NOTED OTHERWISE. WHERE SPlice LENGTHS ARE NOT DIMENSIONED ON THE DRAWINGS, SEE THE FOLLOWING TABLE FOR DIMENSIONS:

MINIMUM TENSION LAP SPlice				
BAR	25MPa (mm)	30MPa (mm)	35MPa (mm)	40MPa (mm)
10M	400	400	400	400
15M	600	550	500	450
20M	750	700	650	600
25M	1200	1100	1000	950
30M	1400	1300	1200	1150
35M	1700	1500	1400	1300

- 2.9 DOWELS SHALL BE SPLICED BEFORE CONCRETE FOOTINGS ARE POURED. TEMPLATES SHALL BE USED TO ENSURE CORRECT PLACEMENT OF DOWELS. DOWELS TO MATCH VERTICAL BARS.
- 2.10 REINFORCEMENT REQUIREMENTS ARE SHOWN ON DETAIL DRAWINGS. WHERE DETAILS OF BAR SIZING AND SPACING ARE NOT SHOWN, ALLOW FOR MINIMUM REINFORCEMENT IN ACCORDANCE WITH CAN/CSA A23.3-04.
- 2.11 ALL HOOKED BARS TO BE STANDARD HOOKS U.N.O. HOOK CAN BE HORIZONTALLY OR VERTICALLY INSTALLED.
- 2.12 PROVIDE SUFFICIENT CHAIRS AND SUPPORT BARS TO MAINTAIN COVER AS SPECIFIED, AND TO MAINTAIN REINFORCEMENT STEEL SECURELY IN PLACE DURING CONCRETE PLACEMENT.
- 2.13 ALL REBAR AT THE END OF FOUNDATION ELEMENTS TO END WITH A STANDARD HOOK.



Chalten Engineering

EGBC PP: 1000485

DESCRIPTION/ GENERAL NOTES

ALL MEASUREMENTS MUST BE CHECKED ON SITE AND BE VERIFIED BY THE CONTRACTOR. DO NOT SCALE OFF HARD COPY DRAWINGS OR ANY ELECTRONIC/COMPUTER FILES. WRITTEN DIMENSIONS ALWAYS HAVE PRECEDENT. HARD COPY DRAWINGS ARE THE OFFICIAL DOCUMENTS FOR THE PROJECT AND ALWAYS TAKE PRECEDENT OVER ALL ELECTRONIC/COMPUTER FILES. COPYRIGHT RESERVED. THIS DRAWING AND DESIGN IS AND AT ALL TIMES REMAINS THE EXCLUSIVE PROPERTY OF CHALTEN ENGINEERS AND CANNOT BE USED WITHOUT THEIR WRITTEN CONSENT AND ASSOCIATED DISCLAIMER FEES. THE INFORMATION CONTAINED HERE IN IS FOR USE OF THE CLIENT ONLY.

REFERENCE DRAWINGS

TITLE:

GENERAL NOTES - PAGE 1

REVISIONS DETAIL	DATE
REV. ISSUED FOR BP	29.JAN2025
REV.	
REV.	
REV.	

PROJECT

WHISTLER ART RUN CENTER  
5678 ALTA LAKE ROAD  
WHISTLER BC

JOB N°: 2025XXX SCALE: 1/4"=1'-0" DRAWN: ST DESIGNED: SG CHECKED: SG DATE: 29JAN2025 DRAWING N°	REV./VERSION: <div>A</div> SIZE: D
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S001



F. ROUGH CARPENTRY:

1. GENERAL

1.1 PROVIDE STRUCTURAL WOOD FRAMING/AND DECKING AND PERFORM WORK ACCORDING TO 2018 B.C. BUILDING CODE AND CAN/CSA-086.1-09.

1.2 PREPARE AND SUBMIT FOR REVIEW SHOP DRAWINGS OF ALL STRUCTURAL ASSEMBLIES AND COMPONENTS.

2. PRODUCTS:

2.1 LUMBER SHALL CONFORM TO CAN/CSA-0141-05, THE NLGA STANDARD GRADING RULES FOR CANADIAN LUMBER, AND SHALL HAVE A MAXIMUM 15% MOISTURE CONTENT AT TIME OF INSTALLATION.

2.2 SIZES: ALL MEMBER DESIGNATIONS ARE ACTUAL FINISHED DIMENSIONS.

2.3 LUMBER GRADE: No 1 OR BETTER SPF UNLESS NOTED OTHERWISE FOR ALL MEMBERS DETAILED ON STRUCTURAL DRAWINGS INCLUDING JOISTS, PLATES, STUDS, LEDGERS, AND BCKING. SOLID POSTS AND BEAMS TO BE DOUGLAS FIR #1 - TOP AND BOTTOM PLATES D.FIR #2 OR BETTER.

2.4 PLYWOOD: DOUGLAS FIR SHEATHING GRADE ACCORDING TO CSA 021 (LATEST) EXTERIOR GRADE.

2.5 ALL NAILS TO BE COMMON NAIL SIZES IN LENGTH AND DIAMETER AS SHOWN UNLESS NOTED OTHERWISE:

NAIL LENGTH (MM)	51	64	76	89	101
NAIL DIAMETER (MM)	2.84	3.25	3.66	4.47	4.88

2.6 THIN GAUGE GUN NAILS AND NOTCHED HEAD NAILS ARE NOT ACCEPTABLE. SAMPLES OF GUN NAILS SHALL BE REVIEWED BY THE ENGINEER BEFORE USE. NAIL SPACING SPECIFIED MAY REQUIRE ADJUSTMENT TO SUIT PROPOSED NAIL TYPE.

3. EXECUTION

3.1 ERECT FRAMING TO BC BUILDING CODE, PART 9, SECTION 9.23 "WOOD FRAME CONSTRUCTION" AS A MINIMUM STANDARD, AND AS NOTED ON DRAWINGS AND SPECIFICATIONS.

3.2 ALL NEW BLOCKING TO BE MIN. 38mm THICKNESS, HEIGHT TO SUIT U.N.O. ON DRAWINGS.

3.3 ALL NAILS SHOWN ON DETAILS ARE 76mm LONG NAILS U.N.O.

3.4 ALL FRAMING ANCHORS, METAL STRAPS AND SDS SCREWS TO BE SIMPSON STRONG-TIE PRODUCTS SUBMIT SUBSTITUTE PRODUCT CATALOGUE TO ENGINEER OF RECORD FOR APPROVAL BEFORE ORDERING.

3.5 PRE-DRILLED HOLES ON METAL PLATE OR ANGLES TO MATCH SDS SCREWS. PRE-DRILLED HOLES CAN NOT BE BIGGER THAN 6mm. CENTER SCREWS ON BLOCKING.

3.6 ALL SIMPSON STRONG-TIE CONNECTORS TO BE INSTALLED AS PER MANUFACTURER'S REQUIREMENTS.

3.7 FINGER JOINTED STUDS SHALL NOT BE USED AT HOLD DOWN ANCHORS OR HOLD DOWN STRAPS AS LOCATED ON PLAN.

3.8 REJECT ANY LUMBER WITH SPLITS OR CHECKS GREATER THAN 1/16", LOOSE KNOTS, OR TIGHT KNOTS LARGER THAN 1/4 THE MEMBER DEPTH.

3.9 FLOOR JOISTS - 2X10 AT 16" O/C TYPICAL UNLESS NOTED. PROVIDE DOUBLE JOISTS UNDER ALL PARTITION WALLS. FLOOR JOISTS TO HAVE KNOTS ONLY ON THE UPPER SIDE OF THE JOIST. JOISTS WITH KNOTS ON THE BOTTOM (TENSION) FACE SHALL BE REPLACED.

3.10 LINTELS AND BEAMS TO BE CLEAR LUMBER OR WITH TIGHT KNOTS ON THE UPPER SIDE OF THE BEAM ONLY. FOR NAILING AND BOLTING SEE BELOW. PROVIDE ONE TOP PLATE MINIMUM CONTINUOUS OVER BEAMS AND EXTEND 5'-0" BEYOND END OF BEAM TO LAP AND NAIL TO ADJACENT PLATES WITH J" NAILS AT 6" O/C.

3.11 ALL FRAMING, BRIDGING, BLOCKING, NAILING AND OTHER DETAILS NOT SPECIFIED ON DRAWINGS TO CONFORM TO CANADIAN MORTGAGE AND HOUSING CORPORATION STANDARDS, NATIONAL BUILDING CODE OF CANADA, BRITISH COLUMBIA BUILDING CODE, AND GOOD WORK PRACTICE

3.12 LAMINATE STUDS SOLID UNDER ALL BUILT-UP LINTELS AND BEAMS TO MATCH WIDTH OF BEAM (MINIMUM NUMBER OF STUDS TO EQUAL NUMBER OF BEAM LAMINATIONS). TO SUPPORT ENGINEERED WOOD BEAMS, PROVIDE ONE ADDITIONAL STUD (MINIMUM NUMBER OF STUDS TO MATCH WIDTH OF BEAM PLUS ONE STUD). ENSURE THAT POSTS AND COLUMNS ARE CONTINUOUS THROUGH ALL FLOORS TO FOUNDATIONS WITH SOLID BLOCKING WITHIN FLOOR LEVELS.

3.13 PROVIDE FULL BEARING UNDER ALL POSTS. PROVIDEADDITIONAL BLOCKING AS REQUIRED TO CARRY VERTICAL LOADS THROUGH TO THE FOUNDATION WITH THE BEARING AREA GREATER THAN THE AREA OF THE POST.

3.14 PROVIDE DOUBLE BOTTOM PLATES FOR AT THE BASE OF ALL WALLS WHERE CONNECTION IS MADE TO FOUNDATIONS.

3.15 SHEATHING U/N  
ROOF 5/8" PLYWOOD, 3" COMMON NAILS @ 6" EDGES & 12" INTERIOR  
WALLS 1/2" PLYWOOD, 3" COMMON NAILS @ 6" EDGES & 12" INTERIOR  
FLOORS 5/8" T&G D.FIR PLYWOOD GLUE & NAIL, 3" COMMON NAILS @ 6" EDGES & 12" INTERIOR

3.16 ENSURE TIGHT JOINTS BETWEEN T&G SHEETS FLOOR SHEATHING TO BE TONGUE-AND-GROOVE COFI PLYWOOD SHEATHING PLACED WITH GRAIN PERPENDICULAR TO JOISTS AND WITH STAGGERED JOINTS.

3.17 PROVIDE BRIDGING BETWEEN JOISTS AT 7'-0" CENTERS MAXIMUM AND UNDER ALL PARTITION WALLS PERPENDICULAR TO FRAMING DIRECTION. USE ONLY 2X2 DIAGONAL CROSS BRACING WITHIN THE SPAN. IF SOLID BRIDGING IS INDICATED, PROVIDE AS ADDITIONAL TO CROSS BRIDGING. PROVIDE SOLID BLOCKING AT ALL SUPPORTS BETWEEN JOISTS. PROVIDE A MINIMUM OF 2 TOE NAILS FROM EACH BLOCKING MEMBER TO THE TOP PLATE EXCEPT AT SHEAR WALLS WHERE A MINIMUM OF 4 TOE NAILS ARE REQUIRED EACH BLOCKING MEMBER.

3.18 STUDS AND JOISTS TO ALIGN AT BEARING WALLS. PROVIDE TWO 2X TOP PLATES LAPPED 5'-0" MINIMUM AND NAILED WITH 3" NAILS AT 6" O/C STAGGERED ALONG LAP.

3.19 STEEL CONNECTORS FOR WOOD MEMBERS SHALL BE CSA APPROVED COMMERCIALLY AVAILABLE PIECES WITH RATED LOAD CAPACITIES. ALL METAL CONNECTORS, NAILS, BOLTS, AND OTHER METAL PRODUCTS IN POTENTIALLY MOIST CONDITIONS OR IN CONTACT WITH ANY PRESERVATIVE TREATED WOOD MUST BE EITHER HOT DIP GALVANIZED (G 185 MINIMUM) OR STAINLESS STEEL (TYPE 304 OR 316) AS A MINIMUM. ALL COMPONENTS OF A CONNECTOR INCLUDING FASTENERS SHALL BE OF THE SAME PROTECTION SYSTEM. FOR ADDITIONAL REQUIREMENTS FOR CONNECTORS IN CONTACT WITH PRESERVATIVE TREATED WOOD, SEE METAL CONNECTORS IN TREATED WOOD SECTION.

3.20 PROVIDE JOIST OR BEAM HANGERS FOR ALL FLUSH CONNECTIONS. PRESSURE BLOCKING IS NOT PERMITTED.

3.21 PROVIDE POST BASE AND POST CAP SUPPORTS FOR ALL COLUMNS AND BUILT-UP POSTS WHICH ARE NOT WITHIN WALLS

3.22 BUILT-UP BEAMS:  
2-PLY: 3" COMMON NAILS - 3 ROWS AT 12" ONE SIDE  
3-PLY: 3" COMMON NAILS - 3 ROWS AT 12" EACH SIDE  
4-PLY: 3" COMMON NAILS - 3 ROWS AT 12" TO PREVIOUS PLY  
5 OR MORE: 3" COMMON NAILS - 3 ROWS AT 12" TO PREVIOUS PLY PLUS 1/2"

MACHINE BOLTS C/W WASHERS - ONE TOP AND BOTTOM ADJACENT ANY BEAM CONNECTOR OR POST ABOVE PLUS ONE TOP AND BOTTOM AT SUPPORT. BOLTS AT SUPPORT REQUIRED ONLY FOR BEAMS OVER 3-PLY.

BLOCK NARROW BEAMS TO FULL WIDTH OF STUD WALL. ALL BEAMS TO BE FRAMED FLUSH WITH TOP OF JOISTS UNLESS NOTED ON PLANS OR OTHERWISE APPROVED BY CHALTEN ENGINEERING. PROVIDE ADDITIONAL 2-2X10 DROPPED MINIMUM DIRECTLY OVER WINDOW DOOR HEADERS UNLESS NOTED OTHERWISE ON PLANS

3.23 BUILT-UP STUD POSTS:  
2-PLY: 3" COMMON NAILS - 3 ROWS AT 12" ONE SIDE  
3-PLY: 3" COMMON NAILS - 3 ROWS AT 12" EACH SIDE  
4-PLY: 3" COMMON NAILS - 3 ROWS AT 12" TO PREVIOUS PLY  
5 OR MORE: 3" COMMON NAILS - 3 ROWS AT 12" TO PREVIOUS PLY PLUS 1/2"  
MACHINE BOLTS C/W WASHERS - ONE TOP AND BOTTOM AND AT 6' -0" MAX CENTERS.

POSTS OVER 8'-0" HIGH PROVIDE 1/2" MACHINE BOLTS C/W WASHERS TOP AND BOTTOM AND AT 6'-0" CENTERS OVER FULL HEIGHT OF POST.

3.24 STEEL MEMBERS USED IN FRAMING SHALL FIT TIGHT TO WOOD MEMBERS, BE SHOP PRIMED WITH RUST INHIBITIVE PAINT, AND BE CO-ORDINATED WITH THE ARCHITECTURAL FINISHES TO ENSURE NO ENCROACHMENT UPON FINAL FINISHED SURFACES.

3.25 MAXIMUM HEIGHT FOR INTERIOR NON-LOAD BEARING 2X4 INTERIOR PARTITION WALLS IS 14'-6". PROVIDE 2X6 IF WALL ABOVE THIS HEIGHT. MAXIMUM HEIGHT FOR INTERIOR NON-LOAD BEARING 2X6 INTERIOR PARTITION WALLS IS 23'-0". 22. PROVIDE PRESERVATIVE TREATED LUMBER WHERE CONSTRUCTION IS OUTDOOR OR IN POTENTIALLY MOIST LOCATIONS OR IN DIRECT CONTACT WITH CONCRETE OR MASONRY EXTERIOR WALLS. CRAWLSPACE PONY WALLS SHALL HAVE PRESERVATIVE TREATED SILL PLATES. SEE ITEM 15 ABOVE.

3.26 GLULAM AND PARALLAM FABRICATION TO CONFORM TO CSA 0122 AND SHALL BE MANUFACTURED IN PLANTS CERTIFIED BY CSA 0177. DESIGN TO CONFORM TO CAN3-086.

3.27 GLULAM BEAMS TO BE 24F-E SINGLE BENDING GRADE FOR SINGLE SPAN BEAMS AND 24F-EX DOUBLE BENDING GRADE FOR MULTIPLE SPAN BEAMS. GLULAM COLUMNS TO BE 24F-EX DOUBLE BENDING GRADE. GLULAM BEAMS AND COLUMNS TO BE EXTERIOR SERVICE GRADE. ARCHITECTURAL FINISH GRAD. MEMBERS TO BE STAMPED WITH MANUFACTURER'S IDENTIFICATION MARK ON TOP FACE. ALL HARDWARE HANGERS, CONNECTORS, ETC. REQUIRED FOR GLULAM COLUMNS AND BEAMS TO BE SPECIFIED AND SUPPLIED BY GLULAM MANUFACTURER. SHOP DRAWINGS AND ERECTION DRAWINGS ARE REQUIRED FOR ALL GLULAMS, GLULAM HARDWARE, HANGERS, CONNECTORS, ETC. SUBMIT SHOP DRAWINGS AND ERECTION DRAWINGS TO CHALTEN ENGINEERING FOR REVIEW PRIOR TO FABRICATION.

3.28 GLULAM, PARALLAM, AND MICROLAM IF ANY TO BE DELIVERED AND ERECTED WITH WATERPROOF WRAPPING IN PLACE. MAINTAIN WEATHERPROOF COVERING UNTIL BUILDING IS ENCLOSED.

3.29 GLULAM, PARALLAM, AND MICROLAM IF ANY TO BE CONNECTED WITH BOLTED OR LAGGED CONNECTIONS WITH STEEL CONNECTOR PLATES UNLESS NOTED. PROVIDE STEEL CONNECTORS WITH RATED LOAD CAPACITIES FOR THE LOADS GIVEN ON THE DRAWINGS. FOR CONNECTIONS WITH NO LOADS INDICATED, SUPPLY CONNECTORS WITH CAPACITIES EQUAL TO OR GREATER THAN THE SHEAR CAPACITY OF THE SUPPORTED MEMBER. SPECIALLY FABRICATED CONNECTORS TO BE SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN BRITISH COLUMBIA.

3.30 INSTALL ALL WEYERHAEUSER PRODUCTS IF ANY IN STRICT ACCORDANCE WITH WEYERHAEUSER SPECIFICATIONS. MAINTAIN A DRY ENVIRONMENT FOR ALL WEYERHAEUSER PRODUCTS.

3.31 WEYERHAEUSER TO PROVIDE DETAILS OF REQUIRED BLOCKING, BRIDGING AND CONNECTION DETAILS FOR WEYERHAEUSER PRODUCTS.

3.32 MICROLAM OR PARALLAM BEAMS IF ANY AS MANUFACTURED BY WEYERHAEUSER STRUCTURAL PROPERTIES AS SUPPLIED AND WARRANTED BY THE MANUFACTURER. MICROLAM TO BE MINIMUM 1 BE MICROLAM LVL AND PARALLAMS TO BE A MINIMUM 2 OE PARALLAM PSL.

3.33 ENGINEERED WOOD FLOORS IF ANY ARE DESIGNED USING WEYERHAEUSER PRODUCTS. IF A SPECIFIED MEMBER IS NO LONGER AVAILABLE, SUBSTITUTE A MEMBER OF CURRENT SPECIFICATION WITH EQUAL OR GREATER STRENGTH AND STIFFNESS.

3.34 MICROLAM BEAMS OVER 2 LAMINATIONS TO BE BOLTED WITH 3/4" DIA. MACHINE BOLTS ALTERNATE TOP AND BOTTOM AT 16" O/C IN ADDITION TO NAILING AS SPECIFIED BY TRUS JOIST.

3.35 BOLTS TO BE A307 MACHINE BOLTS WITH WASHERS EACH END IN TIGHT FIT DRILLED HOLE.

3.36 REDI-ROD TO BE A MINIMUM OF 36 KSI YIELD STEEL. MATERIALS CONSULTANT TO TEST A RANDOM SAMPLING OF ROD TO CONFIRM CAPACITY OF 36 KSI ON NET AREA.

G. LETTERS OF ASSURANCE:

LETTERS OF ASSURANCE FOR THE DESIGN OF PRIMARY AND SECONDARY STRUCTURAL COMPONENTS BY SPECIALTY STRUCTURAL ENGINEERS AND OTHER REGISTERED PROFESSIONALS SHALL BE OF THE FORM "SCHEDULE S SPECIALTY ENGINEER ASSURANCE OF PROFESSIONAL DESIGN AND FIELD REVIEW" AS PROVIDED BY THE ASSOCIATION OF PROFESSIONAL ENGINEERS AND GEOSCIENTISTS OF BRITISH COLUMBIA. LETTERS OF ASSURANCE SHOULD BE ANNOTATED TO REFER SPECIFICALLY TO THE AREA OF WORK ENCOMPASSED BY THE REGISTERED PROFESSIONAL. LETTERS OF ASSURANCE BY SPECIALTY STRUCTURAL ENGINEERS ARE REQUIRED FOR:

1. ALL CASES WHERE PROFESSIONAL ENGINEERING IS REQUIRED FOR STRUCTURAL OR SEMI-STRUCTURAL ELEMENTS OF THE BUILDING.

2. MATERIALS CONSULTANT TO CONFIRM THAT MATERIALS FOR THE PROJECT CONFORM WITH APPLICABLE CODES AND SPECIFICATIONS FOR THE BUILDING.

3. SPECIALTY STRUCTURAL ENGINEERS FOR DESIGN AND FIELD REVIEW OF BUILDING ELEMENTS OF COMPONENTS THAT FORM PART OF THE FINAL PRIMARY STRUCTURE INCLUDING THOSE INDICATED IN THE FOLLOWING NOTES.

4. SPECIALTY STRUCTURAL ENGINEERS FOR DESIGN AND FIELD REVIEW OF SECONDARY STRUCTURAL ELEMENTS OF THE BUILDING REQUIRED TO FACILITATE THE ARCHITECTURAL DESIGN OF THE BUILDING.

5. SPECIALTY STRUCTURAL ENGINEERS FOR TEMPORARY WORKS SUCH AS BUILDING STABILITY, FORMWORK, RESHORING, COMPONENT TEMPORARY STABILITY OR STRENGTH, CONFIRMATION OF TEMPORARY LOADS ON THE STRUCTURE, CONSTRUCTION LOADS ON THE STRUCTURE, SEQUENCING OF CONSTRUCTION, ETC. WHETHER ON BEHALF OF THE OWNER OR THE CONTRACTOR(S).

6. SPECIALTY STRUCTURAL ENGINEERS FOR SECONDARY AND SEMI-STRUCTURAL ELEMENTS AND ARCHITECTURAL ELEMENTS INCLUDING BUT NOT LIMITED TO STEEL STUDS, SEISMIC TIES FOR BRICK WORK, STONE TIES, ARCHITECTURAL PRECAST, GLAZING, HANDRAILS, ESCALATOR DESIGN AND CONNECTION, WINDOW WASHING ANCHORAGE AND SYSTEMS, ELEVATOR DIVIDER BEAMS, AND SIMILAR ITEMS AS REQUIRED TO FACILITATE THE BUILDING DESIGN.

7. SPECIALTY ENGINEER FOR ARCHITECTURAL COMPONENTS SUCH AS CEILING TIES, BOOK SHELVING, LOCKERS, AND ANY OTHER MISCELLANEOUS ARCHITECTURAL ITEMS THAT MAY AFFECT SAFETY OF OCCUPANTS.

8. GEOTECHNICAL CONSULTANT FOR ALL SOILS AFFECTS INCLUDING SOILS LATERAL LOADING TO THE STRUCTURE, COMPACTION, LIQUEFACTION, BEARING CAPACITY, PASSIVE RESISTANCE, ETC. AS IT RELATES TO BOTH THE TEMPORARY CONSTRUCTION CONDITION OF THE BUILDING AND TO THE FINAL COMPLETED BUILDING.

9. SPECIALTY STRUCTURAL ENGINEERS COMMONLY KNOWN AS "SEISMIC CONNECTION ENGINEERS" FOR THE DESIGN AND FIELD REVIEW OF THE ANCHORAGE AND RESTRAINT FOR GRAVITY, WIND AND SEISMIC LOADING INCLUDING DETAILED DESIGN OF THE CONNECTION TO THE PRIMARY STRUCTURAL ELEMENT OF THE BUILDING AS DETERMINED BY THE STRUCTURAL ENGINEER OF RECORD.

10. REGISTERED PROFESSIONALS PROVIDING SERVICES TO THE PROJECT AFFECTING OR FORMING PART OF THE PRIMARY STRUCTURE OF THE BUILDING.

11. REGISTERED PROFESSIONALS AS MIGHT BE REQUIRED BY THE OWNER OR OTHER CONSULTANTS OR THE AUTHORITY HAVING JURISDICTION.

THE PRIMARY PURPOSE OF THESE LETTERS IS TO VERIFY FULL AND COMPLETE SERVICE BY THE SPECIALTY CONSULTANT. THE EXTENT AND DEPTH OF SERVICE REQUIRED SHALL BE CONSISTENT WITH THAT OF THE STRUCTURAL ENGINEER OF RECORD. FULL TIME SUPERVISION OF THE DESIGN OR WORK IN THE FIELD IS NOT REQUIRED EXCEPT AS DEEMED NECESSARY BY THE SPECIALTY ENGINEER IN THE CIRCUMSTANCES. THE ENGINEER IS RESPONSIBLE FOR CONDUCTING THE NECESSARY FIELD REVIEWS FOR HIS WORK AND SHALL APPLY HIS PROFESSIONAL DISCRETION WITH REGARD TO THE EXTENT OF FIELD REVIEW AND REVIEW AT FABRICATION PLANTS. IN ADDITION, AND NOT TO ANNUL OR DIMINISH THE RESPONSIBILITY OF THE SPECIALTY CONSULTANT, THE STRUCTURAL ENGINEER OF RECORD MAY REQUIRE ADDITIONAL SITE REVIEW BY THE SPECIALTY CONSULTANT OR ALTERNATIVE PARTIES AS HE MAY DEEM SUITABLE. REGARDLESS OF SUCH ADDITIONAL REVIEW, THE FULL ENGINEERING RESPONSIBILITY FOR THE SPECIALTY ELEMENTS SHALL BE BORNE BY THE SPECIALTY CONSULTANT ALONE.

SEE OTHER CONSULTANTS FOR REQUIREMENTS FOR LETTERS OR ASSURANCE PERTAINING TO STRUCTURAL OR NON-STRUCTURAL BUILDING COMPONENTS PERTAINING TO THE WORK OF OTHER CONSULTANTS OR THE OWNER.

H. SPECIALTY STRUCTURAL ENGINEER:

12. A SPECIALTY STRUCTURAL ENGINEER IS AN ENGINEER REGISTERED IN BRITISH COLUMBIA, EMPLOYED BY A CONTRACTOR(S), THE OWNERS, OR OTHERS, WHO POSSESSES SPECIALTY KNOWLEDGE OR EXPERIENCE AND IS RESPONSIBLE FOR DESIGN AND FIELD REVIEW OF SPECIFIC COMPONENTS OF THE PRIMARY STRUCTURE AND/OR THE OTHER SEMI-STRUCTURAL BUILDING COMPONENTS, WHETHER FORMING PART OF THE STRUCTURE OR AS PART OF THE WORK SPECIFIED BY OTHER CONSULTANTS ON THE PROJECT. WORK BY A SPECIALTY STRUCTURAL ENGINEER IS INDEPENDENT OF THE STRUCTURAL ENGINEER OF RECORD.

I. STRUCTURAL STEEL

1. PRODUCTS:

1.1 STRUCTURAL STEEL SHALL CONFORM TO THE GENERAL REQUIREMENTS OF CAN/CSA-G40 20-04 AND CAN/CSA-G40 21-04. ALL STEEL WORKS TO BE IN ACCORDANCE WITH CBC 2018 AND CSA STANDARD S16-09.  
1.2 STRUCTURAL STEEL: PLATES, CHANNELS AND ANGLES: GRADE 300.  
1.3 THREADED RODS AND ANCHOR RODS: ASTM F1554.  
1.4 HOLLOW STRUCTURAL SECTIONS CLASS C & ROLLED SECTIONS - GRADE 350W  
1.5 ALL BOLTS SHALL BE GALVANIZED HIGH STRENGTH STRUCTURAL BOLTS CONFORMING TO ASTM A325 TYPE 1 20mm DIAMETER MINIMUM, U.N.O. ON DRAWINGS.  
1.6 ALL WEATHER EXPOSED STEEL TO BE HOT DIP GALVANIZED TO CAN/CSA-G164.  
1.7 ALL WELDING ELECTRODES: E49XX  
1.8 PROVIDE SHOP DRAWINGS SIGNED AND SEALED BY P. ENG.REGISTERED IN BC OF ALL STRUCTURAL STEEL AND MISCELLANEOUS METAL FOR REVIEW, PRIOR TO FABRICATION. SHOP DRAWING TO SHOW ALL DETAILS AND MATERIAL SPECIFICATION.  
1.9 REQUIREMENTS BY CSA W59 MUST BE FULFILLED BY FABRICATORS CERTIFIED TO CSA W47.1 DIV. 1 OR DIV. 2.  
1.10 NO BURNING OF HOLES SHALL BE PERMITTED IN STRUCTURAL STEEL.  
1.11 PRIMER: CISC/CPPMA 2-75 FOR INTERIOR NOT EXPOSED STEEL.

M. STRUCTURAL STEEL

2. EXECUTION:

2.1 ALL WELDING TO BE IN ACCORDANCE WITH C.S.A. W59. FABRICATION SHOP TO HAVE FULL APPROVAL OF C.W.B. TO THE REQUIREMENTS OF C.S.A. W47.1 DIVISION 1 OR 2. ALL WELDERS TO BE CERTIFIED.

2.2 WELDING TESTING REQUIRED:  
FULL PENETRATION WELDS : 100%  
FILLET WELDS: 25%

2.3 CONTRACTOR MUST NOTIFY THE ENGINEER OR OWNER REPRESENTATIVE WELL IN ADVANCE OF THE TIME WELDING WILL BE READY FOR QUALITY ASSURANCE TESTING. WELDS MUST REMAIN UNCOVERED AND ACCESSIBLE UNTIL RESULTS FROM TESTING AGENCIES ARE SUBMITTED & APPROVED BY ENGINEER.

2.4 DO NOT FIELD BURN BASE PLATE HOLES OR CONNECTION BOLT HOLES. IF BOLT HOLES ARE MISALIGNED, INFORM ENGINEER.

2.5 STRUCTURAL STEEL CONTRACTOR TO PROVIDE ALL TEMPORARY BRACING REQUIRED TO STABILIZE THE STEEL FRAME UNTIL THE STRUCTURE IS COMPLETE.

2.6 PROVIDE SHOP DRAWINGS FOR ALL STRUCTURAL STEEL.

2.9 STEELWORK EXPOSED TO THE EXTERIOR TO BE PAINTED AS FOLLOWS, UNLESS NOTED OTHERWISE:  
2.9.1 SURFACE PREPARATION TO SSPC-SP-6. COMMERCIAL BLAST.  
2.9.2 COAT (5 MILS, THICKNESS) OF MARINE ALKYD METAL PRIMER (MASTER PAINTERS INSTITUTE PRODUCT NO.79). PRIMER TO BE COMPATIBLE WITH FINISHED PAINT SPECIFICATION. SEE ARCHITECTURAL FOR FINISHED PAINTING.  
2.9.3 FIELD TOUCH UP ABRASIONS AND PAINT BOLTS.

2.10 SITE WELDS AND THE SURROUNDING AFFECTED AREA SHALL BE CLEANED BY WIRE BRUSH AND TOUCHED UP WITH ZINC RICH PRIMER. MINIMUM DRY THICKNESS TO BE 50 MICRONS.

2.11 CONTRACTOR HAS THE OPTION TO SPLIT STEEL MEMBERS INTO SMALLER PIECES TO FACILITATE TRANSPORTATION AND ERECTION. FULL STRENGTH SPlicing CONNECTION IS REQUIRED TO SPLICE THE INDIVIDUAL PIECES ON SITE TO FORM A COMPLETE MEMBER. FULL STRENGTH SPlicing CONNECTION IS AT THE CONTRACTOR'S OWN COST.

2.12 APPLY AT LEAST ONE COAT OF ZINC PRIMER TO MINIMUM DRY FILM THICKNESS OF 50 TO 75 MICRONS.

2.13 ALL PIECES OF STEEL IN CONTACT WHERE WELDS ARE NOT SHOWN SHALL BE WELDED TO 100% OF TENSION CAPACITY. WHERE WELDS OR CONNECTION FORCES ARE SHOWN, DESIGN FOR THAT VALUE.

2.14 ALL EXTERIOR HSS SECTIONS SHALL HAVE CAP PLATES SEAL WELDED ALL AROUND.

2.16 IF WELDING IS REQUIRED CONTRACTOR IS ALLOWED TO CHOOSE SHOP WELD OR FIELD WELD FOR ALL WELDS REQUIRED. ANY CALL OUTS ARE SHOWN ON DRAWINGS ARE FOR REFERENCE ONLY AND ARE ENGINEER'S SUGGESTION.

2.17 ALL EXPOSED WELDS SHALL BE GROUND SMOOTH.



Chalten Engineering

EGBC PP: 1000485

DESCRIPTION/ GENERAL NOTES

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REFERENCE DRAWINGS

TITLE:

GENERAL NOTES - PAGE II

REVISIONS DETAIL	DATE
REV. ISSUED FOR BP	29.JAN2025
REV.	
REV.	
REV.	

PROJECT

WHISTLER ART RUN CENTER  
5678 ALTA LAKE ROAD

WHISTLER BC

JOB N°: 2025XXX SCALE: 1/4"=1'-0" DRAWN: ST DESIGNED: SG CHECKED: SG DATE: 29JAN2025 DRAWING N°	REV./VERSION: <div>A</div> SIZE: D
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S002



DESIGN LOADS	
ROOF	
SNOW	177 PSF
DEAD	15 PSF
STAIRS	
DEAD	15 PSF
LIVE	100 PSF

LANDING JOIST SCHEDULE (HANGER)	
MARK	DESCRIPTION
J01	2X6 AT 16 (LU26L)

BEAM LANDING SCHEDULE (HANGER)	
MARK	DESCRIPTION
BL1	3-2X8 (HUC HANGER)

JOIST SCHEDULE (HANGER)	
MARK	DESCRIPTION
JR1	2X6 AT 16 (LRU26Z)
JR2	2X6 AT 16 (LRU26Z)

ROOF BEAM SCHEDULE (HANGER)	
MARK	DESCRIPTION
BR1	4X10 D FIR

LEDGER	
MARK	DESCRIPTION
LR	2X6 C/W 2-3/8 GRK'S AT 16" TO EXISTING FRAMING

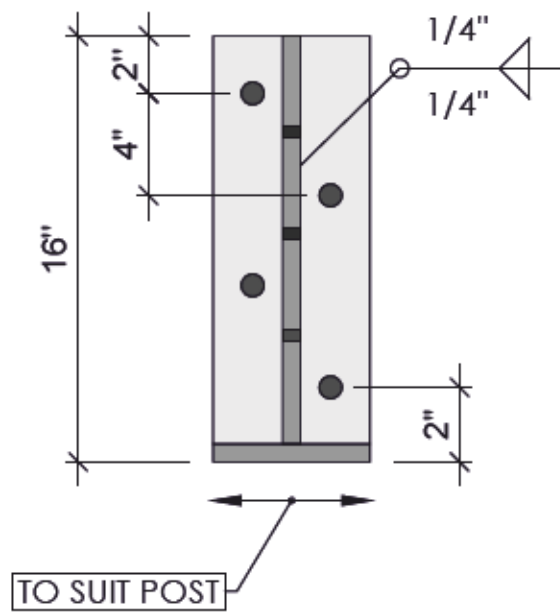
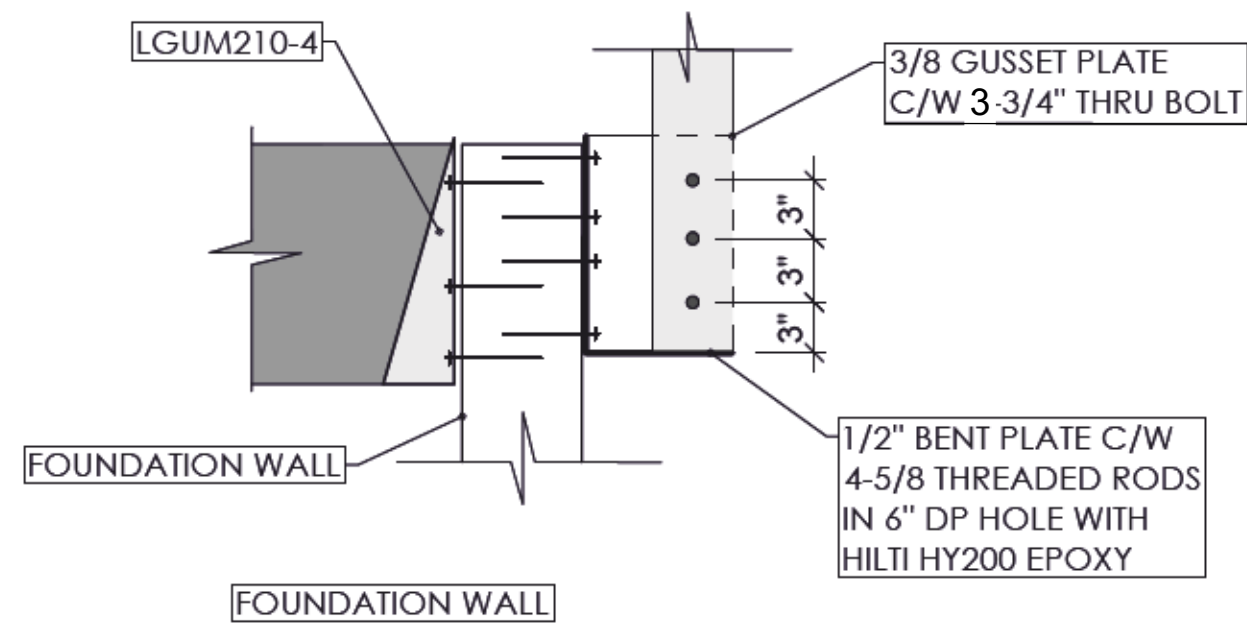
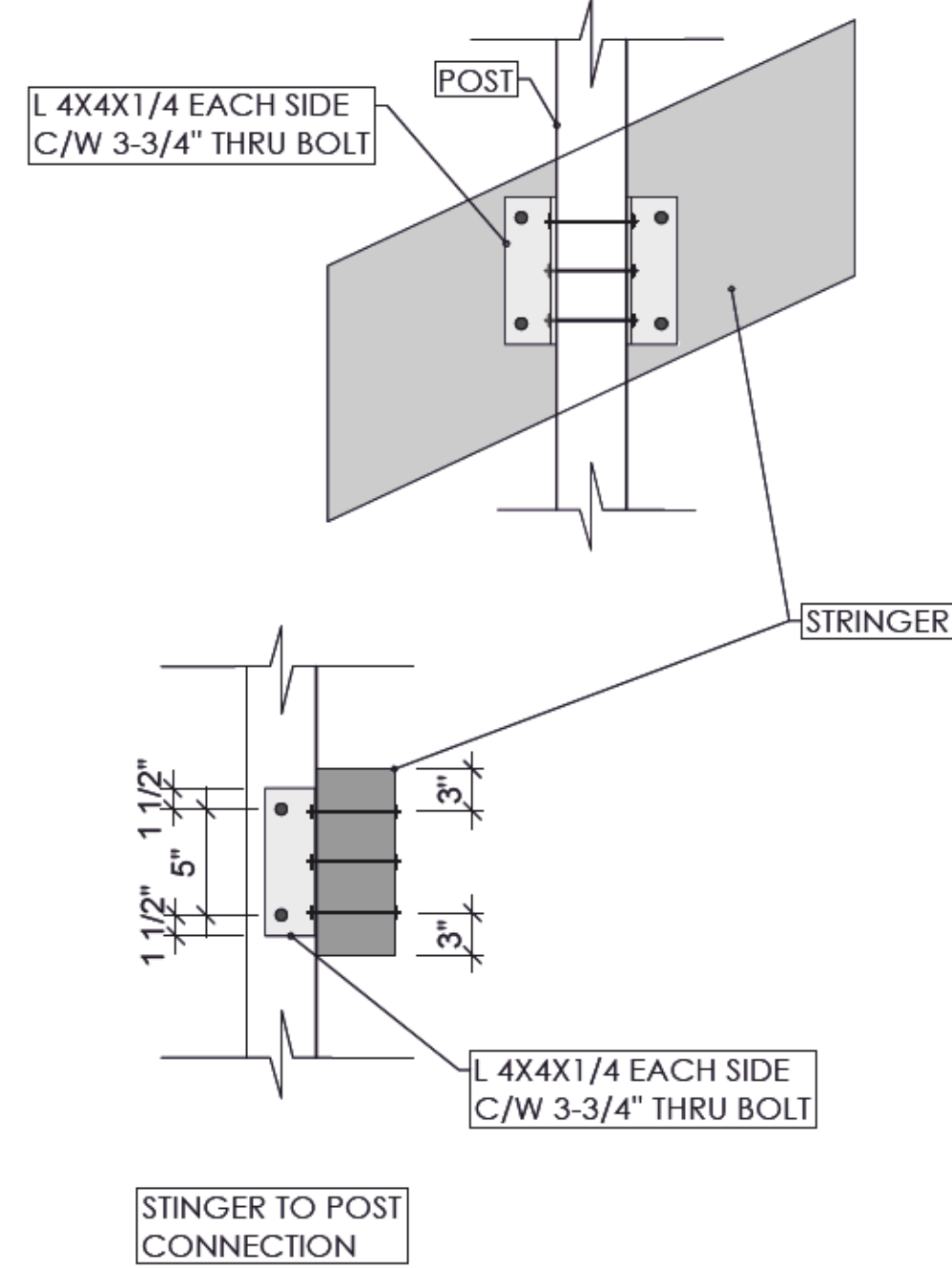
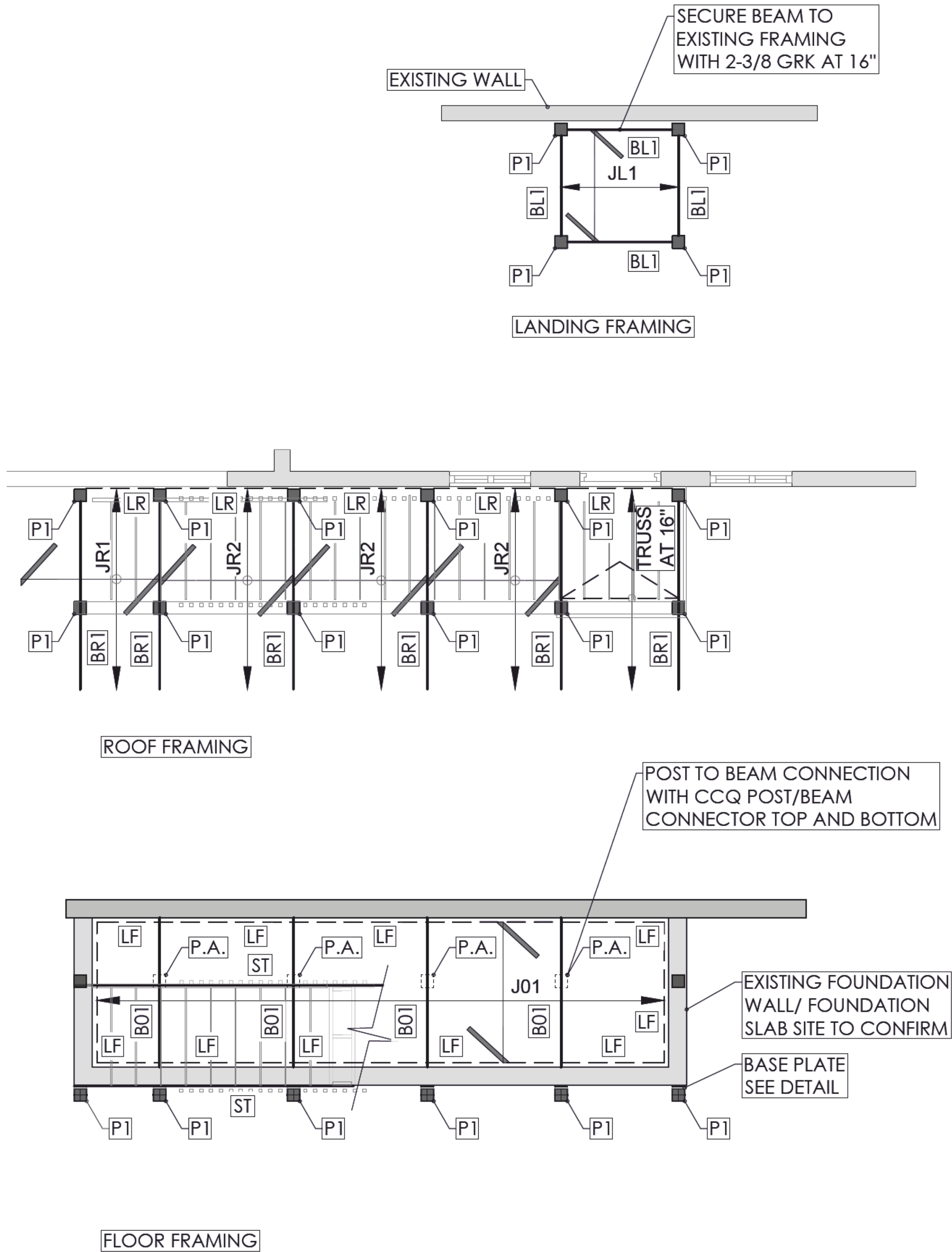
POST SCHEDULE	
MARK	DESCRIPTION
P1	3 1/8 X 6" GLULAM OR 6X6 D FIR

FLOOR BEAM SCHEDULE (HANGER)	
MARK	DESCRIPTION
B01	4-2X10 D FIR (LGLUM210-4)
ST	4X12 STRINGER

FLOOR JOIST SCHEDULE (HANGER)	
MARK	DESCRIPTION
J01	2X6 AT 16 (LU26L)

LEDGER	
MARK	DESCRIPTION
LF	2X6 C/W 5/8 AT 16" HILTI HAS ROD IN 4 1/2" DP HOLE WITH HILTI HY200

SHEATHING	
3/4" PLYWOOD SHEATHING C/W 2 1/2" NAILS AT 6" O/C EDGE-12" O/C FIELD	



DESCRIPTION/ GENERAL NOTES

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REFERENCE DRAWINGS

TITLE:

GENERAL PLANS & DETAILS

REVISIONS DETAIL	DATE
REV. ISSUED FOR BP	29JAN2025
REV.	
REV.	
REV.	

PROJECT

WHISTLER ART RUN CENTER  
5678 ALTA LAKE ROAD  
WHISTLER BC

JOB N°: 2025XXX SCALE: 1/4"=1'-0" DRAWN: ST DESIGNED: SG CHECKED: SG DATE: 29JAN2025	REV./VERSION: <div>A</div> SIZE: D
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DRAWING N°

S010